

Artigo

Coupling WRF and NRCS-CN Models for Flood Forecasting in Paraíba do Meio River Basin in Alagoas, Brazil

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Abstract

Coupling the WRF and NRCS-CN models was assessed as a tool for a flood forecast system. The models were applied to the Paraíba do Meio River basin, located in Alagoas, Brazil. FNL (Final Analysis GFS) data provided by the Global Forecast System model were used as initial conditions for WRF. Precipitations and observed discharges were collected in data collection platforms. Nine microphysics configurations were used to optimize WRF forecast. For hydrological, the automatic calibrations, available in HMS was used to get the optimum CN model parameters. Optimized precipitations Model performance was assessed with the indicators: bias, root-mean-square error, Pearson's linear correlation coefficient, Nash-Sutcliffe coefficient, Heidke skill score, hit rate and false alarm rate. WRF's predictive ability for the optimum configuration was satisfactory. The NRCS-CN yielded good results. The predictive ability of the hydrological model was ranked between satisfactory and acceptable. In a flood forecasting step, the coupled model yielded Nash-Sutcliffe of 0.749 and 0.572 for Atalaia and Viçosa basins. Overall, the method showed potential for the development of a flood alert system.

Keywords: WRF model, NRCS-CN model, short-term rainfall forecasting, flood forecast.

Acoplamento dos modelos WRF e NRCS-CN para previsão de cheias na bacia do rio Paraíba do Meio em Alagoas, Brasil

Resumo

O acoplamento dos modelos WRF e NRCS-CN foram avaliados como ferramentas para um sistema de previsão de cheias. Os modelos foram aplicados na bacia hidrográfica do rio Paraíba do Meio, localizada em Alagoas, Brasil. FNL (Final Análises GFS) dados obtidos do Sistema de Previsão Global foram utilizados como condições iniciais para o WRF. Precipitações e vazões observadas foram coletadas das plataformas de observação de dados. Nove configurações de microfísica foram usadas para otimizar as previsões do WRF. Para o modelo hidrológico, foram utilizadas calibrações automáticas disponíveis no HMS. Foram otimizados os parâmetros do modelo NRCS-CN. O desempenho dos modelos foi avaliado com os indicadores: viés, mínimo erro quadrático, coeficiente de correlação linear de Pearson, coeficiente de Nash-Sutcliffe, Heidke skill score, acertos e alarmes falsos. A habilidade de previsão do WRF para a configuração ótima foi considerada satisfatória. O modelo NRCS-CN gerou bons resultados de cheias. A habilidade preditiva do modelo hidrológico variou de satisfatória a aceitável. Na etapa de previsão de cheias, o modelo acoplado gerou coeficientes de Nash-Sutcliffe de 0.749 e 0.572 para as bacias Atalaia e Viçosa. Em seu todo, o modelo acoplado apresentou um bom potencial para desenvolvimento de sistemas de alerta.

Palavras-chave: modelo WRF, modelo CN-NCRS, previsão de cheias, previsão de chuvas de curto tempo.

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1. Introduction

Many studies have been done coupling meteorological to hydrologic models for improving decisions on water resources management and planning. Thom et al. (2017) used gridded rainfall products in SWAT model, to estimate discharges in Srepok River Catchment in Vietnam. Givati et al. (2012) coupled the WRF model to the Hydrological Model of Karst Environment (HYMKE) for predicting streamflow operationally for the high Jordan River Basin. Shrestha et al. (2014) used data from downscaling from of the HadCM3 global circulation model as input for the hydrological model in HEC-HMS to study the impacts of climate change in Kulekhani Hydropower Project in Nepal. Alves et al. (2012) coupled a regional atmospheric spectral model (RSM) to SMAP hydrological model for defining reservoir release rules in semiarid region of Brazil.

The coupling of forecasting precipitation models with hydrological models with a short-term horizon is fundamental for developing flood alert systems in urban and rural areas. The prediction of floods and the issuance of warnings to populations can save human and animal lives and reduce damage. Many studies have focused on the improvement of coupled models and their assessments (Chen *et al.*, 2011; Linares-Rodriguez *et al.*, 2015; Yang *et al.*, 2015; Shahid *et al.*, 2017; Ratna *et al.*, 2017).

The coupling of climate and hydrological models can be unidirectional (off-line) or bidirectional (online). In unidirectional coupling, meteorological data predicted using an atmospheric model and rainfall and evapotranspiration are used as input data in a hydrological model. This coupling offers improved flexibility and operational autonomy. In bidirectional coupling, there is an exchange of data between meteorological and atmospheric models, resulting in an integrated model called a hydrometeorological model. The incompatibilities of time and spatial scales are considered the major problems in bidirectional coupling. Nevertheless, bidirectional coupling has great potential for flood forecasting (Yu *et al.*, 1999).

This method was applied by Meller *et al.* (2014) for flood forecasting in the Paraopeba River basin, located in the state of Minas Gerais, using the Model for Large Basins of the Hydraulics Research Institute - MGB-IPH) and rainfall forecasts developed at the University of São Paulo. The results showed promise for identifying and predicting floods. Calvetti and Pereira Filho (2014) applied the WRF model coupled to TopModel, in hourly time step, for streamflow prediction in Iguaçu river basin in southern Brazil. The authors found better results using more complex microphysics schemes.

In this paper, the coupling of the WRF atmospheric model with the NRCS-CN hydrological model using the software HMS (NRCS: Natural Resources Conservation Service; CN: Curve Number; HMS: Hydrological Model Systems) was analyzed for developing a flood forecasting and alert system for the Paraíba do Meio River. To improve our knowledge of atmospheric models, nine microphysics and convection configurations were assessed for the WRF model. In addition to the coupling of the atmospheric and hydrological models, this study is innovative in that it yielded an optimal microphysics and convection configuration for the WRF coupled with optimal NRCS parameters in the coast of Northeastern Brazil. Unidirectional coupling was adopted due to its potential for providing flowrate forecasts and its easy model coupling.

2. Study Area

The hydrographic basin of the Paraíba do Meio River (HBPM) is located between latitudes 8°44' and 9°44' south and longitudes 36°48' and 35°52' west. It measures 3,148.5 km2, of which 1,964.66 km2 is in the state of Alagoas and 1,183.8 km² in the state of Pernambuco (Fig. 1).

The HBPM is underlain by two geological domains. Crystalline soils prevail in the upstream portion, and sedimentary soils prevail in the downstream portion. The elevation of the basin ranges from 1,024 m at the riverhead to 1.0 m at the river's mouth. The relief ranges from rugged to undulatory in the upper valley, in the region of the Borborema Plateau. The lower valley is characterized by smooth relief in the region of the coastal tablelands and in the fluvial-lagoonal plains.

The head of the Paraíba do Meio River is located in the city of Bom Conselho, state of Pernambuco, at an elevation of 800 m. The river empties into the Mundaú-Manguaba estuarine lagoon complex in the city of Pilar, on the coast of the state of Alagoas. The river generally flows southeast, is 172 km long, and has a perennial fluvial regime.

The HBPM is located in the eastern part of northeastern Brazil. In this region, 60% of the rainfall occurs during the four months of April to July. The mean annual rainfall ranges from 1,300 mm at the coast to 700 mm at the riverhead (Rao *et al.*, 1993; Vitorino *et al.*, 1997).

The following synoptic systems act in the region: cold fronts and their remnants (Kousky, 1979), waves from the east (Yamazaki and Rao, 1977), VCANs (Gan and Kousky, 1986), CCMs (Alves, 2001) and wavelike disturbances of the trade winds. The number of rainy days ranges from 70 to 120 (Silva *et al.*, 2012).

3. Data and Methods

3.1. Data

Two series of rainfall and flowrate data, collected at data collection platforms (DCPs) (Table 1), were used. The rainfall series spanned a 120-hour period from July 27



Figura 1 - Location of the Paraíba do Meio River basin, in Alagoas, Brazil.

to 31, 2011, and the series used in the validation phase spanned 192 h from June 1 to 8, 2013. The flowrate series used in the calibration and validation of the hydrological model contained 120 records (July 1 to 5, 2013) and 192 records (July 9 to 16, 2013), respectively. For the hydrological modelling, the rainfall and flowrate data were stored in the data storage system (DSS) of the Hydrologic Engineering Center (HEC). The spatial distribution in the basin was obtained using the Thiessen polygon.

As initial boundary conditions for the WRF, data in a $1.0^{\circ} \times 1.0^{\circ}$ grid were operationally prepared every six hours (12 a.m., 6 a.m., 12 p.m. and 6 p.m. Universal Coordinated Time (UTC). The data were provided by the National Center for Environmental Prediction (NCEP) of

the Final Operational Global Analysis (FNL). The FNL data were generated using the same model used by the NCEP in the Global Forecast System (GFS) (Almeida and Marton 2014).

3.2. WRF atmospheric model

The WRF is a cutting-edge numerical atmospheric model developed by several research centers and government agencies in the United States, including the National Center for Atmospheric Research (NCAR), Mesoscale and Microscale Meteorology Division of the National Oceanic and Atmospheric Administration (NOAA), the National Center for Environmental Prediction (NCEP), and the

 Table 1 - DCPs used for the calibration and validation of the WRF and SCS/HMS models.

Code	Station type	Latitude	Longitude
936119	Rainfall	-9 22' 45.12"	-36 14' 57.12"
39710000	Flowrate		
936110	Rainfall	-9 30' 4.12"	-36 1' 22.08"
39870000	Flowrate		
	Code 936119 39710000 936110 39870000	CodeStation type936119Rainfall39710000Flowrate936110Rainfall39870000Flowrate	Code Station type Latitude 936119 Rainfall -9 22' 45.12" 39710000 Flowrate 936110 Rainfall -9 30' 4.12" 39870000 Flowrate

Source: National Water Agency.

Naval Research Laboratory (NRL) (Skamarock *et al.* 2005).

The WRF was set up with three integration domains: domain D1, with 81 x 81 points in the xy directions, a 25km grid spacing, and coverage of all of northeastern Brazil; domain D2, with 91 x 91 points in the xy directions and a horizontal grid spacing of 5 km; and domain D3, with 151 x 151 points in the xy directions and a horizontal grid spacing of 1 km.

To find the best configuration for the simulation, the nine microphysics and convection parametric schemes listed in Table 2 were evaluated. The results simulated with the WRF in domains D2 and D3 in two sub-basins, Atalaia (sub-basin SB12) and Viçosa (sub-basin SB9), and their upstream contribution areas were evaluated. No significant differences were observed between the results in domains D2 and D3; therefore, to reduce the computational effort, domain D2 was selected.

With the combinations of the microphysics and convection schemes, a matrix with nine elements was created (Table 3). The schemes referring to the surface boundary layer, soil surface layer, planetary boundary layer and atmospheric radiation were specified.

The WRF model was executed using the configuration listed in Table 4. The simulation spanned the 120 h from July 27, 2011, at 00Z, to July 31, 2011, at 00Z.

3.3. NRCS-CN hydrological model

Estimation surface runoff from rainfall data is of major importance in hydrological engineering and watershed management. Among the various methods available, the NRCS-CN methodology is widely accepted and popular (Verma *et al.*, 2017). For the hydrological modelling using NRCS-CN, HEC-HMS software was used. The

 Table 2 - Parametric schemes evaluated in the calibration of the WRF model.

Scheme	Notation
Kessler (1969) – microphysics	MPK
Lin et al. (1983) – microphysics	MPL
Thompson et al. (2004) – microphysics	MPT
Kain and Fritsch (1993) – convection	KF
Betts and Miller (1986) and Janjić (1994) - convection	BMJ
Grell and Dévényi (2002) - convection	G3D

 Table 3 - Possible combinations of the three microphysics schemes and three convection schemes in the WRF model.

	KF	BMJ	G3D
MPK	MPK-KF	MPK-BMJ	MPK-G3D
MPL	MPL-KF	MPL-BMJ	MPL-G3D
MPT	MPT-KF	MPT-BMJ	MPT-G3D

 Table 4 - Configuration of the WRF model using the parametric schemes.

Configuration	Scheme	Reference
Microphysics	Purdue Lin	Lin et al., 1983
Cumulus	Grell 3D	Grell and Dévényi, 2002
Surface boundary layer	MM5 Similarity	Beljaars, 1995
Soil surface layer	Unified Noah Land Surface Model	Tewari <i>et al.</i> , 2004
Planetary boundary layer	Asymmetric Convection Model	Pleim, 2007
Atmospheric radia- tion	Goddard Shortwave	Chou and Max, 1994
	RRTMG Longwave	Iacono <i>et al.</i> , 2008

HMS allows the simulation of many hydrological processes in a hydrographic basin. The loss function was used to estimate the fraction of rainfall that converts to direct surface runoff. The transformation function used data from a hyetograph and made it possible to obtain the hydrograph at a control point in the basin. The HMS provided an automatic calibration tool to estimate the parameters of the hydrological model. The HMS is widely used in association with other HEC software to study floods in urban centers, flood frequency and flood losses (Singh and Woolhiser, 2002). In addition, the HMS is a multi-model program that allows the user to develop the most appropriate model for the analyzed system

For the loss function, the curve number (CN; NRCS, 1986) was applied. The method uses the CN coefficient, which depends on the use and type of soil. The exceeding rainfall was estimated using Eq. (1):

$$Pe = \begin{bmatrix} \frac{(P-I_a)^2}{P-I_a+S} & if \ (P-I_a) > 0\\ 0 & elsewhere \end{bmatrix}$$
(1)

where Pe is the effective rainfall of the event (mm), P is the total rainfall of the event (mm), Ia corresponds to the initial losses (mm), and S is the potential maximum retention of rainfall (mm).

The value of Ia was estimated using Eq. (2):

$$Ia = 0.2S(2)$$
 (2)

Combining Eqs. (1)-(2) yields Eq. (3):

$$Pe = \begin{bmatrix} \frac{(P - 0.2S)^2}{P + 0.8S} & if \ (P - 0.2S) > 0\\ 0 & elsewhere \end{bmatrix}$$
(3)

The value of S can be calculated as a function of the CN using Eq. (4):

$$S = \frac{25400}{CN} - 245 \tag{4}$$

The CN ranges from 0 to 100 and varies as a function of the soil group and use, the occupation and the initial moisture condition. The CN values were tabulated by the NRCS (NRCS). The estimated CN can be refined by performing the automatic calibration available in the HMS model, which provides 14 objective functions for the calibration.

For converting the exceeding rainfall to a flowrate, the NRCS unit hydrograph method was used (Soil Conservation Service, 1972). It is one of the most widely used models in practice, due to its simplicity and ease of application (Milde *et al.*, 2002).

3.4. Analysis of model performance

The performance of the model was assessed in the WRF configuration phase, in the WRF/HMS coupling and in flood forecasting. In the configuration and coupling phases, the following metrics were used: the bias (Eq. (5)), the root-mean-square error (*RMSE*; Eq. (6)) and the correlation coefficient r (Eq. (7)). These three equations are as follows:

$$Bias = \frac{1}{N} \sum_{i=1}^{N} (M_i - O_i)$$
(5)

where M_i is the *i*-th value obtained from the modeling (rainfall or flowrate), O_i is the value observed at the surface (rainfall or flowrate), and N is the number of data analyzed.

$$RMSE = \sqrt{\left(\sum_{i=1}^{N} \left(M_i - O_i\right)^2\right)/N}$$
(6)

where M_i is the *i*-th value estimated by the model, O_i is the value observed at the surface, and N is the number of data analyzed.

$$r = \frac{Cov(O_I, M_I)}{S_{M_i} \cdot S_{O_i}}$$
(7)

where *r* is the linear correlation coefficient between O_i and M_i ; and S_{M_i} and S_{O_i} are the standard deviations of M_i and O_i , respectively.

For the evaluation of the predictive ability, the Nash-Sutcliffe (*NS*) coefficient, the Heidke skill score (*HSS*), the hit rate (*H*) and the false alarm rate (*FAR*) were used.

The *NS* values for the WRF model and for the coupled models were calculated using Eq. (8):

$$NS = 1 - \frac{\sum_{i=0}^{n} (O_i - M_i)^2}{\sum_{i=0}^{n} (O_i - \overline{O})^2}$$
(8)

where O_i is the observed value, P_i is the simulated value, and \overline{O} is the mean observed value; all variables may be for the rainfall or flowrate.

For rainfall, the *HSS*, estimated using Eq. (9), was applied by assuming two conditions: rainfall and no rainfall.

$$HSS = \frac{2(xw - yz)}{[(x + z) \cdot (z + w) + (x + y) \cdot (y + w)]}$$
(9)

where x indicates that the model predicted rainfall and that rainfall occurred, y indicates that the model predicted rainfall but that rainfall did not occur, z indicates that the model did not predict rainfall but that rainfall occurred, and w indicates that the model did not predict rainfall and that rainfall did not occur.

The value of H (dimensionless) was estimated using Eq. (10):

$$H = \frac{x+w}{n}.100\tag{10}$$

where n indicates the number of observations (x and w were defined earlier).

The value of FAR was estimated using Eq. (11):

$$FAR = \frac{y}{x+y} \tag{11}$$

(*y* and *x* were defined earlier).

For the flood discharges, for alert purposes, the *HSS* was applied to three conditions: low flowrate or a normal condition, average flowrate or a watch condition, and a high flowrate or a warning condition. The *HSS* was calculated using Eq. (12):

$$HSS = \frac{\frac{1}{N} \sum_{i=1}^{k} N(F_i O_i) - \frac{1}{N^2} \sum_{i=1}^{k} N(F_i) N(O_i)}{1 - \frac{1}{N^2} \sum_{i=1}^{k} N(F_i) N(O_i)}$$
(12)

where $n(F_i,O_j)$ is the number of forecasts in category *i* that corresponded to observations in category *j*, $N(F_i)$ is the number of forecasts in category *I*, $N(O_j)$ is the number of observations in category *j*, and *N* is the total number of forecasts. Table 5 presents the categories used to determine the *HSS*.

The *HSS* values can range from -1 to 1, where the value of 1 indicates a perfect forecast; zero indicates no predictive ability or a forecast equivalent to a reference forecast, i.e., a fortuitous coincidence; and -1 indicates performance inferior to a random forecast.

			Observed cat	egory		
Predicted category	i <i>/j</i>	Class 1	Class 2		Class k	Total
	Class 1	$n(F_1, O_1)$	$n(F_1, O_2)$		$\mathbf{n}(F_1, O_k)$	$N(F_1)$
	Class 2	$n(F_2, O_1)$	$n(F_2, O_2)$		$n(F_2, O_k)$	$N(F_2)$
	Class k	$n(F_k, O_1)$	$n(F_k, O_2)$		$n(F_k, O_k)$	$N(F_K)$
	Total	$N(O_1)$	$N(O_2)$		$N(O_K)$	Ν

Table 5 - HSS multi-category contingency table.

4. Results and Discussion

4.1. Atmospheric modeling

4.1.1. Calibration of the parametric configuration

Table 6 presents the results of the nine combinations corresponding to Atalaia station. At this station, a negative bias prevails in the predictions by the WRF model, given that eight of the nine combinations yielded negative values. Combination MPK-KF yielded the lowest bias (-0.01 mm/h). The second-lowest biases were those produced by combinations MPT-KF (-0.07 mm/h) and MPL-KF (0.07 mm/h). Combination MPT+KF produced the lowest RMSE (1.55 mm/h) and the highest r (0.68 mm/h). The correlation obtained in the analysis (0.68 mm/h) was acceptable and was rated as strong based on the classification of Callegari-Jacques (2003). Thus, combinations MPT-KF performed best among the nine combinations: it performed best based on two indicators and second best based on the third indicator.

At Viçosa station, the WRF model captured the rainfall events well, although it underestimated the rainfall intensities there much more than it did at Atalaia station. The area scale factor used for the mean rainfall could be responsible for the lower quality of this forecast. The Viçosa area is much smaller than the Atalaia area. Table 7 shows that all the biases are negative, and the lowest value was -0.42 mm/h.

 Table 6 - Statistics for the simulations of the nine combinations evaluated using the WRF-ARW model, Atalaia station.

Combination	r	Bias	RMSE
MPK-KF	0.55	-0.01	1.82
MPK-BMJ	0.31	-0.84	2.17
MPK-G3D	0.53	-0.55	1.87
MPL-KF	0.63	0.07	1.66
MPL-BMJ	0.44	-0.81	2.08
MPL-G3D	0.46	-0.27	2.03
MPT-KF	0.68	-0.07	1.55
MPT-BMJ	0.38	-0.82	2.12
MPT-G3D	0.61	-0.28	1.69

 Table 7 - Statistics of the simulation of the nine combinations evaluated in the WRF-ARW model, Viçosa station.

Combination	r	Bias	RMSE
MPK-KF	0.42	-0.97	2.05
MPK-BMJ	0.36	-0.93	2.04
MPK-G3D	0.29	-0.61	2.80
MPL-KF	0.46	-1.02	2.08
MPL-BMJ	0.42	-0.95	2.03
MPL-G3D	0.42	-0.50	2.19
MPT-KF	0.21	-0.99	2.14
MPT-BMJ	0.20	-0.95	2.14
MPT-G3D	0.40	-0.42	1.89

H values exceeding 50% were obtained, except with combination MPT-KF, which yielded values of 44.54% and 42.86% for the Atalaia and Viçosa stations, respectively. The analysis revealed that the best H was produced by combination MPL-G3D: approximately 80%.

For FAR values, the worst result was produced by combination MPT-KF, which yielded values of 55.46% and 53.21% for Atalaia and Viçosa stations, respectively. The best result was produced by combination MPK-BMJ, which yielded FARs of 21.43% and 13.95% for Atalaia and Viçosa stations, respectively.

Eight of the nine combinations yielded positive HSSs, indicating performance better than a random forecast. The highest HSS values, 0.5817 for Atalaia station and 0.5319 for Viçosa station, corresponded to combination MPL-G3D. The worst result was obtained with combination MPT-KF, which yielded values of 0 (zero) and -0.0836, indicating that this combination is inadequate for rainfall forecasting in this region.

Based on these results, combination MPT-KF performed poorly. In contrast, the adjustments represented by combinations MPT-G3D and MPL-G3D provided the best rainfall simulations in the Paraíba do Meio River basin. Combination MPL-G3D was selected for use in the WRF model, given that it provided the best overall results.

4.1.2. Validation - application of the selected configuration

For the validation of the WRF model, simulations spanning a 192-h time period were performed, i.e., 72 h longer than that of the configuration (calibration). Table 8 presents the results of the statistical analysis.

The RMSE obtained from the validation was within acceptable limits despite the high value corresponding to Atalaia station. HSS values of 0.37 and 0.61 were obtained for Viçosa and Atalaia stations, respectively (Table 8).

4.2. Hydrological modelling

The NRCS-CN hydrological model was calibrated using, as input, a time series of rainfalls with 120 records from the period of July 1 to 5, 2013. The basin was subdivided into 14 sub-basins (Fig. 2). Two flowrate stations were used in the calibration: the DCP of the city of Viçosa, located in sub-basin SB9, and the DCP of Atalaia (subbasin SB12).

4.2.1. Calibration of the hydrological model

For the calibration, manual and automatic searches for the best NRCS-CN parameters were performed. The manual method consisted of varying the parameters until

Table 8 - Statistical evaluation of the validation of the WRF model.

Station	r	Bias (mm/h)	RMSE (mm/h)	HSS
Atalaia	0.32	0.07	10.45	0.61
Viçosa	0.15	-0.09	2.92	0.37



Figura 2 - Division of the Paraíba do Meio basin into sub-basins for simulation in the HEC/HMS.

an optimal response was produced based on the judgment of the analyst. Due to its simplicity, this method is widely applied. During each trial, the adjustment of the maximum flowrates, the shape of the calculated hydrograph, the adjustment of the flood peaks and the calculated volume were analyzed. The CNs and response times (RTs) were thus calibrated.

The automatic calibration consisted of using the search tool available in the HEC-HMS. The parameters of the NRCS-CN hydrological model were adjusted to minimize the percent error peak objective function available in the HMS.

The results of the calibration are presented in Table 9. The adjustment provided simulated flowrates very close to the observed values. Based on the calibration, the predictive ability of the model was rated as adequate and good according to the classification developed by Motovilov *et al.* (1999). The mean NS coefficient of 0.822 between two points of analysis was obtained; a higher value was obtained for sub-basin SB12 (0.836). The relation between the simulated and observed series indicated a very strong correlation, with a mean coefficient of 0.921 and a median amplitude between the simulated and observed flowrate in the calibration phase; however, the flood peaks were satisfactorily simulated.

The hydrographs in Fig. 3 allow for a comparison between the observed and simulated series. Good fits of the flood peaks can be observed.

In addition, in the calibration phase, one of the most important aspects of a simulation of the flowrate for an alert system is the prediction of the hydrographs and the sizes of the peaks. Therefore, an adjustment that yielded better results in terms of these two properties was sought.

Meenu *et al.* (2013) calibrated the HEC-HMS in an evaluation of the impacts of climate changes in the hydrographic basin of the Tunga-Bhadra River in India. The results were statistically compatible with those obtained in this study. The consistency tests yielded satisfactory predictive ability, with an NS coefficient of 0.48 and r equal to 0.85. According to Oleyiblo and Li (2010), despite the simple structure of the HEC-HMS, when calibrated, the

 Table 9 - Statistical criteria evaluated in the calibration of the HMS parameters with respect to observed rainfall in sub-basins SB9 and SB12.

Statistical criterion	Sub-basin SB9	Sub-basin SB12
	Calibrated	Calibrated
NS	0.809	0.836
R	0.905	0.938
RMSE	0.073	1.067
Bias	-0.184	-5.837



Figura 3 - Observed and simulated flowrates (observed rainfall) in the NRCS/HMS calibration phase, with the contribution of rainfall throughout the basin and with analysis point at the DCPs of sub-basins SB9 (a) and SB12 (b). The rainfall event spanned from July 1 to 5, 2013.

software becomes a powerful flood forecasting tool. Roy *et al.* (2013), in a forecasting study performed in eastern India, obtained an NS coefficient of 0.72.

4.2.2. Validation of the hydrological model

The validation consisted of a model simulation using the parameters following their adjustment during another series of observed flowrates and rainfall amounts. The selected period spanned 192 hours from July 9 to 16, 2013. The results are presented in Table 10.

The calibrations for sub-basins SB12 and SB9 yielded NS coefficients of 0.809 and 0.591, respectively. Both values are satisfactory, particularly the value for SB12. The r values were 0.945 and 0.908 for SB12 and SB09, respectively, indicating good correlations.

The flowrate peaks were simulated well by the model (Fig. 4). Although the peak in sub-basin SB9 (Fig. 4a) was below the observed value, the predictive ability of the model is considered satisfactory (NS =

 Table 10 - Evaluation of the hydrological model in the validation of the HMS parameters, sub-basins SB9 and SB12.

Statistical criterion	Sub-basin SB9	Sub-basin SB12
	Validated	Validated
NS	0.591	0.809
r	0.945	0.908
RMSE	0.295	1.251
Bias	-2.257	-3.866

0.591), based on the classification of Motovilov *et al.* (1999).

The model underestimated the flowrates, based on the bias. However, low discrepancy between the two series was observed, based on the RMSE. Thus, the validation demonstrates the acceptable calibration of the model. The timing and intensity of the flood peaks were identified satisfactorily.

Given the results of the validation, coupling of the WRF atmospheric model and the NRCS-CN hydrological model was performed to evaluate the technique as a tool for forecasting hydrologic events in the Paraíba do Meio River basin.

4.3. Unidirectional coupling of the hydrological model with the WRF

Following the calibration and validation, the model was used to simulate the flowrate of the Paraíba do Meio River, using the rainfall simulated by the WRF atmospheric model as forcing in the NRCS-CN hydrological model. The grid points generated by the WEF model were selected in the BRPM limits for use as hypothetical pluviometers. The goal of this coupling of the WRF with the NRCS-CN model was to predict the flowrate in the extremely short term. The period of the coupling spanned from July 1 to 5, 2013.

The criteria were used to evaluate the efficacy of the coupling. The results of the evaluation are presented in Table 11. The technique proved to be suitable based on the statistical results. The implication is that the coupling is adequate to satisfactory, based on the NS coefficient. The



Figura 4 - Observed and simulated flowrates (observed rainfall) in the validation of the HMS, with the contribution of the rainfall throughout the basin and with analysis at the DCPs of sub-basins SB9 (a) and SB12 (b). The rainfall event spanned from July 9 to 16, 2013.

Table 11 - Statistical criteria evaluated in the coupling of the SCS/HMS with the WRF in sub-basins SB9 and SB12, period of July 1 to 5, 2013.

Statistical criterion	Sub-basin SB9	Sub-basin SB12
	Coupled	Coupled
NS	0.75	0.57
HSS	0.25	0.73
PA	61%	84%
r	0.76	0.75
$RMSE (m^3/s)$	0.11	2.49
Bias (m ³ /s)	-0.24	13.15

HSS, consistent with the Nash coefficient, indicates that the coupled system performed satisfactorily in sub-basin SB12. The system also yielded PA values of 84% and 61% in the flowrate simulations in sub-basins SB12 and SB9, respectively.

The correlations are 0.76 and 0.75 for sub-basins SB9 and SB12, respectively, indicating a low combined variation between the two series, which, consequently, led to a strong correlation.

Cabral *et al.* (2016) obtained an r of 0.48 for the coupling of the RAMS atmospheric model with the SMA hydrological model of the HEC-HMS when applied to the hydrographic basin of the Alto Jaguaribe River, in the state of Ceará.

In this study, the coupling yielded slight underestimation in sub-basin SB9. The relation between the observed series and the series simulated with the simulated rainfall is considered satisfactory, based on the low RMSE value. In sub-basin SB12, the coupling overestimated the observed flowrates, based on much higher values of the bias and RMSE (Table 11). However, this sub-basin is large, and the bias of 13.15 m3/s is not large in comparison to the flood flowrates, which exceed 100 m^3 /s.

Fig. 5 presents hydrographs of the observed and simulated flowrates. Although the predictions imperfectly simulated the peaks in the hydrographs, the technique was able to capture the flowrate variations in the basin during the period of analysis.

Based on the results, the method exhibits certain limitations, but it has demonstrated its potential as a flood forecasting tool starting with meteorological forecasts. However, further development is still necessary with regard to the configuration of the atmospheric model, given that the meteorological forecasts control the performance of the coupling.

According to Habets *et al.* (2004), despite all breakthroughs in atmospheric modelling, rainfall is still one of



Figura 5 - Observed and simulated flowrates (simulated rainfall) from the coupling of the HMS (hydrological) and WRF (atmospheric) models, with contribution of rainfall throughout the basin and with analysis at the DCPs of sub-basins SB9 (a) and SB12 (b). The rainfall event spanned from July 1 to 5, 2013.

the most difficult variables to predict in that it displays large temporal and spatial variations. Nevertheless, despite the WRF model's great dependency on simulated rainfall values, the coupled model shows promise for the creation of a flood alert system.

5. Conclusions

This study of the configuration of the WRF atmospheric model indicates that the best parametric combination among the microphysics and convection schemes for extremely short-term rainfall forecasting in the Paraíba do Meio River basin are the schemes proposed by Purdue Lin (microphysics) associated with Grell 3D (convection). The model shows satisfactory predictive ability, based on our evaluation of statistical indicators.

The unidirectional coupling (WRF - NRCS-CN) proved to be suitable for extremely short-term flowrate forecasting and for decision-making regarding flood alerts. The coupling yielded correlation coefficients exceeding 0.75. The predictive ability of the coupled system was good, based on NS coefficients of 0.749 and 0.572 for subbasins SB12 and SB9, respectively.

The HSS values corresponding to three flowrate categories (Q \leq 50, 50< Q \leq 100 and Q>100 m³/s) were 0.73 for Atalaia station and 0.25 for Viçosa station. The most severe flooding hazard is in Atalaia, where the upstream discharge area is large (2517.73 km²).

In summary, the unidirectional coupling of the WRF, using the optimized configuration, with the NRCS-CN,

using the optimized hydrological parameters, could serve well in developing flood alert systems.

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