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Change in the dynamics of salinity and water quality of an island estuary by the discharge of effluents

Alteração na dinâmica da salinidade e na qualidade das águas de um estuário insular pelo lançamento de efluentes

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ABSTRACT

Anthropic changes in coastal watersheds affect the quantity and quality of water in estuaries. Based on an analytical model of saline intrusion and load balance, we have evaluated the effects of effluent discharge (mean of 285 L·s⁻¹ and peak of 495 L·s⁻¹) from a wastewater treatment plant (WWTP) in an island estuary. Saline intrusion at low tide, without actual anthropic discharge, will increase by 22%, whereas with increasing discharge, reductions of 16% and 28%. The reduction of saline intrusion may affect biogeochemical processes and the distribution of species in regions further up the estuary. When the WWTP reaches the mean projected discharge, it will increase its BOD and phosphorus loads by 90% and 82%, respectively, in relation to the current load. With the increase of WWTP discharge, there will be an expansion of hypoxic and anoxic conditions over the current areas, worsening the condition of this already jeopardized estuary. For the WWTP to lead to the expected environmental gains, it is necessary to consider the carrying capacity of the receiving body.

Keywords: Estuary; Salt intrusion; Carrying capacity.

RESUMO

As alterações antrópicas nas bacias hidrográficas costeiras afetam a quantidade e a qualidade da água dos estuários. A partir de um modelo analítico de intrusão salina e do balanço de cargas, avaliamos os efeitos da descarga de efluentes projetada para uma estação de tratamento de esgoto (ETE) (média de 285 L·s⁻¹ e pico de 495 L·s⁻¹) em um estuário insular. A intrusão salina na maré baixa, sem descarga antrópica, aumentará em 22%, ao passo que com o aumento da descarga haverá reduções de 16% e 28%. A redução da intrusão salina poderá afetar processos biogeoquímicos e a distribuição de espécies em regiões mais a montante do estuário. Quando a ETE atingir sua vazão média projetada, aumentará suas cargas de DBO e fósforo em 90% e 82%, respectivamente, em relação à carga atual. Com o aumento da vazão da ETE haverá ampliação das atuais áreas em condição de hipóxica e anóxica, piorando a situação atual deste estuário já comprometido. Para que a ETE traga os ganhos ambientais esperados é necessário considerar a capacidade suporte do corpo receptor.

Palavras-chave: Estuário; Intrusão salina; Capacidade suporte.



INTRODUCTION

Aquatic environments in coastal zones (CZ) are under strong pressure from anthropic activities due to the high population density in these areas (Pinto-Coelho & Havens, 2015). The low sanitation rates in the CZ represent one of the main impact factors on the water environment in this region. The estuarine region is a transitional environment between river and sea, being influenced by oceanic and watershed processes, commonly receiving domestic effluents from urban centers. In this context, wastewater treatment plants - WWTP are important structures in urban areas, but they also promote transposition of watersheds, when the supply water comes from a different basin from the one in which the effluent is released, increasing the water input in the receiving body.

Freshwater discharge and seawater inflow, associated with mixing and transport processes in the estuarine mixing zone, provide ideal conditions for the occurrence of biogeochemical processes and sedimentation of suspended particulate matter, increasing the productivity of these environments (Miranda et al., 2002; Bianchi, 2007). Freshwater inputs from anthropogenic sources have the potential to cause changes in the salinity gradient, nutrient, dissolved oxygen, and sediment dynamics in estuarine environments (Kimmerer, 2002), factors that influence the distribution and welfare of estuarine species (Gillson, 2011; Blaber, 2000; Bussell et al., 2008). The salinity gradient in estuaries is an indicator of mixing processes between freshwater from river and saline waters from the adjacent sea, but it is also a key process of saline flocculation. Regions of low salinity favor the flocculation of suspended clay from river drainage, serving as nucleation points for adhesion of inorganic and biological materials, with the growth of the flocs, sedimentation occurs (Bianchi, 2007; Wolanski & Elliott, 2015). The tidal movements in estuaries promote the resuspension of these sediments in certain areas and deposition in others, favoring the absorption of nutrients, metals, and dissolved organic matter present in the water column, representing an efficient process of pollutant removal in estuaries (Stumm & Morgan, 1996; Bianchi, 2007).

Commonly, the self-depuration processes of rivers and estuaries in the CZ are insufficient to absorb the load generated by the anthropic activity. Thus, WWTPs are essential facilities for mitigating the impacts caused by human settlements (Morrison et al., 2011; Cloern et al., 2016). However, depending on the design characteristics of the WWTP and the carrying capacity of the water environment, effluents can bring negative impacts to their receiving bodies, by raising the organic matter and nutrient loads (Haggard et al., 2005; Silva et al., 2016; Cabral et al., 2019). The input of organic matter and nutrients from anthropogenic sources has been a major problem in estuaries around the world (Kennish, 2002; Cloern et al., 2016; Bianchi, 2007). Organic matter, commonly assessed in terms of BOD, in addition to consuming dissolved oxygen in aerobic oxidation process, can also drastically interfere with the community of estuarine organisms, reducing local biodiversity (McLusky & Elliott, 2006). Effluent discharge has drastically affected nutrient fluxes (nitrogen and phosphorus) in aquatic environments (Van Drecht et al., 2009; Mekonnen & Hoekstra, 2018). Causing, by eutrophication, the expansion of the occurrence, intensity, and duration of hypoxic conditions in coastal waters worldwide (Testa & Kemp, 2011), resulting in changes in community structure, loss of biodiversity, and modifications in

the metabolism of the environment (Diaz & Rosenberg, 2008; Scherner et al., 2013). In many urbanized estuaries, effluents are the main sources of nutrients (phosphorus and nitrogen) and organic matter (Pereira Filho & Rörig, 2016; Silva et al., 2016), directly impacting the functioning of the environment and its uses.

In this study, we have a hypothesis that the increase of effluent discharge from a WWTP in an island estuary will promote the retraction of the saline intrusion and reduce the quality of the estuarine water. To evaluate the effect of effluent discharge on water quality, the load balance was used and in the evaluation of saline intrusion from the estuary the model described by Savenije (1986, 2005) was used. This model was applied in 35 estuaries in four continents (Savenije, 2020). However, this study is the first to use this model in Brazil, in an insular estuary of reduced dimensions, with the low fluvial flow, and applied in the evaluation of the reduction of the saline intrusion by the discharge of the effluents of a WWTP.

MATERIALS AND METHODS

Field of study

The Papaquara River watershed, located in the North of Santa Catarina Island, in the city of Florianópolis - SC (Figure 1), has an area of 32 km², having its catchment outlet in the Ratones River, inside the Carijós Ecological Station, protected areas of category full protection (Law 9,985/00 art. 8). The climate of the region is characterized as humid subtropical Cfa (Peel et al., 2007) with rainfall well distributed throughout the year, with the month of February presenting the highest volumes of accumulated rainfall (198 mm, varying 190-210 mm) and June, the lowest volumes (75 mm, varying 70-90 mm), according to Ramos et al. (2009). The region has a semidiurnal astronomical tidal regime with microtidal amplitude with an average of 0.7 m ranging from 0.4 to 1.2 m (Truccolo et al., 2006). The meteorological components have a strong effect on tide, northeastern winds exert an under elevation and south winds over elevation at tide levels, low-frequency south wind events can exert up to 1 m over an elevation on the astronomical tide (Truccolo et al., 2006).

The fixed population estimated in 2017 for Papaquara watershed is approximately 15,500 inhabitants (Prosul, 2012), but this population triples during summer vacation (Guarda, 2012), which starts in the second half of December and lasts until the end of Carnival, the second half of February. The population of Papaquara watershed receives treated water from an aquifer located in another watershed. The public sewage system serves only 0.9 km² of the Papaquara watershed, which corresponds to approximately 22% of the population living there (Figure 1). The remaining population has individual treatment systems (septic systems) with final disposal in the soil and in rainwater drainage system, which represents an estimated flow of 23.5 L·s⁻¹. WWTP currently has an average design capacity of 285 L·s⁻¹ and a maximum hourly flow of 495 L·s⁻¹ (Steinwandter, 2019). The average flow rate of WWTP has its value increased due to the maximum daily flow coefficient and the maximum hourly flow coefficient resulting in maximum hourly flow or peak flow rate (Associação Brasileira de Normas Técnicas, 1986). The operating annual average for 2017

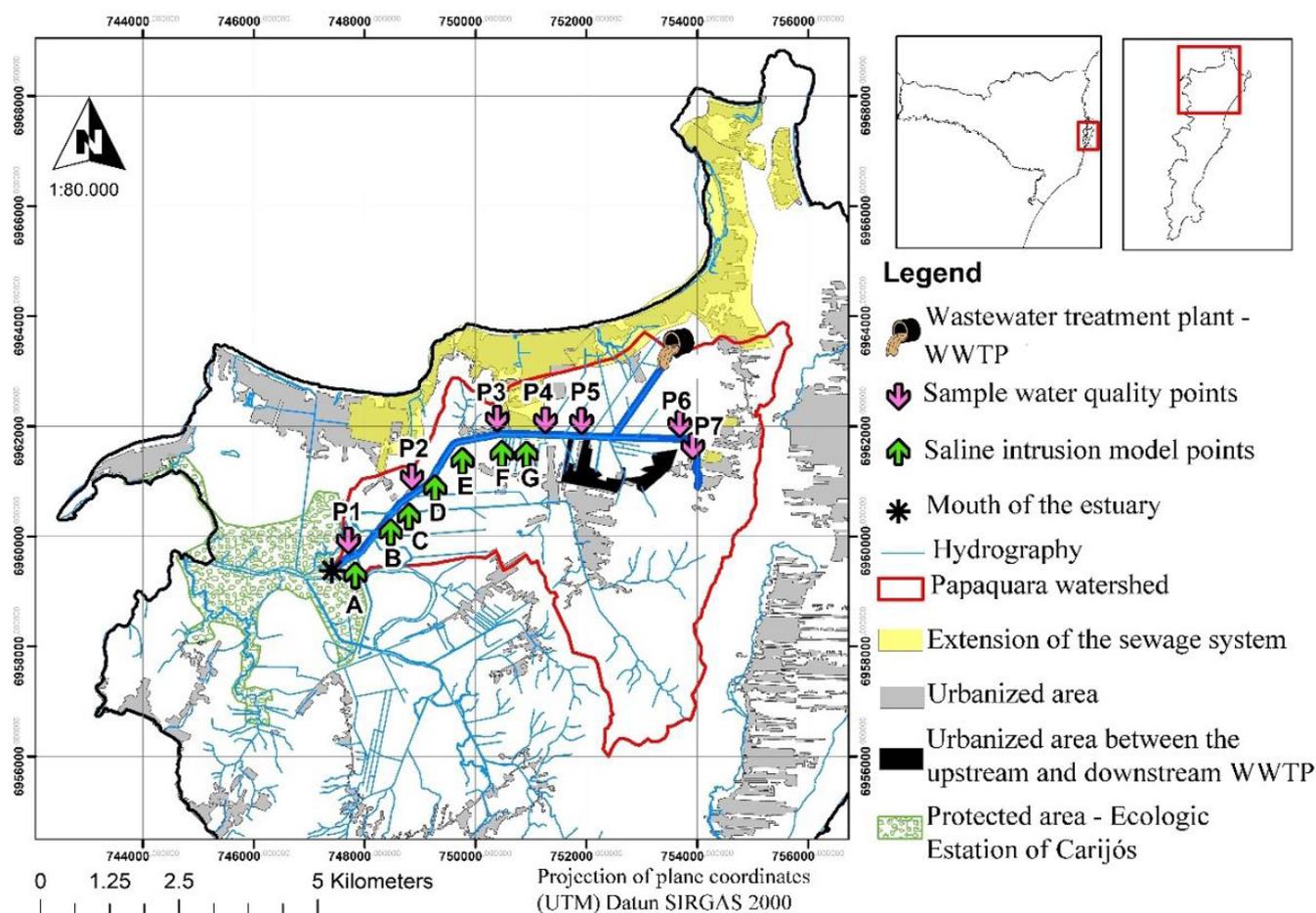


Figure 1. Papaquara River basin, coverage of the public sewage system, urbanized areas, location of the sampling point for water quality assessment, and points for obtaining data from the saltwater intrusion model.

was $135 \text{ L} \cdot \text{s}^{-1}$, with the lowest average recorded in July $95 \text{ L} \cdot \text{s}^{-1}$ and the highest average in January $201 \text{ L} \cdot \text{s}^{-1}$ of the same year (Instituto Chico Mendes de Conservação da Biodiversidade, 2011). The treated effluent is discharged into a drainage channel, which travels 1.8 km until it flows into the Papaquara River (Figure 1). The population covered by WWTP is approximately 40,000, but only 8.5% of this population live in the Papaquara watershed.

Sampling

The analytical model of saline intrusion was generated from data of width, depth (portable echo sounder, Laylin Associates, model SM5), water velocity (flow meter, OTT Hydromet, model OTT MF pro) and calibrated from data of salinity (manual refractometer, Alfakit, model 201bp). The data was obtained in seven sampling stations along the Papaquara River estuary (Figure 1), onboard of a vessel, in the high tide condition associated with the winds of the South quadrant (wind velocity, maximum of 10.9 and average of $1.9 \text{ m} \cdot \text{s}^{-1}$), that promote higher meteorological tide, on August 24, 2015. The average flow for WWTP this day ($85 \text{ L} \cdot \text{s}^{-1}$) was used to calibrate the saline intrusion model. For validation of the model, another water salinity measurements from five sampling date (from October 2013 to April 2014) were carried out at five

sampling station in three tides moments on different days. The average tidal heights at the time of sampling, for validation, were 0.3 m (low tide), 0.5 m (medium tide) and 0.9 m (high tide).

The water quality was evaluated in seven sampling stations along the Papaquara River (P1 to P7), in addition to the location where the final effluent from the WWTP is discharged (Figure 1), during nine sampling campaigns from August 2014 to February 2015. The water quality parameters determined were: salinity (manual refractometer, Alfakit, model 201bp), dissolved oxygen - DO (Associação Brasileira de Normas Técnicas, 1988), biochemical oxygen demand - BOD (Associação Brasileira de Normas Técnicas 1992a), and total phosphorus - TP (Associação Brasileira de Normas Técnicas, 1992b). The percentage of dissolved oxygen saturation (DO%) was calculated as a function of temperature and salinity according to Chapra (1997).

Flow estimate

In the study area, there are no gauging stations, so the river flow was estimated using the equations described in the Santa Catarina State flow regionalization study (Santa Catarina, 2006). The long term average flow rate (Q_{MLT}) or long period flow rate of a river basin corresponding to the average of the annual

average flows or the average of the averages, being estimated by Equation 1 (Santa Catarina, 2006).

$$Q_{MLT} = \left(9.393 \times 10^{-4} \cdot P^{0.362} \cdot AD^{1.092} \right) \cdot 1000 \quad (1)$$

where Q_{MLT} is long-term average flow ($L \cdot s^{-1}$); P is the precipitation ($mm \cdot year^{-1}$) and AD the drainage area (km^2).

The flow-duration curve represents the relations between the flows of a river with the probability that its occurrence equals or exceeds this flow value (Tucci, 2002). The flow-duration curve is obtained by calculating the average long-term flow rate (Equation 1) multiplied by the appropriate percentile coefficient (K_p) for each region (Santa Catarina, 2006).

Garbossa & Pinheiro (2015), when analyzing the uncertainty of the equations proposed by Santa Catarina, (2006) in the continental and island basins that flows into the Santa Catarina Island Bays, found an average deviation of -24% in frequencies close to Q_{50} and 10% in Q_{MLT} for basins with an area above $10.3 km^2$. Despite limitations in the application of the flow regionalization methodology for small basins (Silveira et al., 1998), the equations proposed by Santa Catarina, (2006) are still a good alternative due to lack of continuous monitoring in the region.

Historical annual mean precipitation, of $1.543.9 mm \cdot year^{-1}$ (Ramos, et al., 2009) and the Papaquara watershed area, of $32.0 km^2$, were applied at Equation 1. Also, the values $707 L \cdot s^{-1}$; $496 L \cdot s^{-1}$ and $342 L \cdot s^{-1}$ were considered as Q_{25} , Q_{50} and Q_{75} , respectively. For the estimation of the load water balance in the river, the river flow (Q_{50}) of the Papaquara river was estimated upstream and downstream from the WWTP effluent discharge point; considering a drainage area of $6.1 km^2$ for the upstream point (P6 in Figure 1) and $10.6 km^2$ for the downstream point from the WWTP (P5 Figure 1), the values of Q_{50} were 44.5 and $153 L \cdot s^{-1}$, respectively. The flow of the WWTP was obtained by an ultrasonic level meter installed at the entrance of the raw effluent in the plant (Nivelco brand, model Ecotrek).

Saline intrusion

The one-dimensional analytical model of salinity developed by Savenije (1986, 2005) estimates the longitudinal profiles of salinity in alluvial estuaries, considering geometric parameters of the estuary and its salinity. The main parameters used in the model were detailed in Figure 2.

In a steady-state situation, the partial derivative as a function of time in the salt balance equation is equal to zero. Savenije (2005), considering as constant the fluvial flow (Q_f) and the area of section (A), defined the salt balance equation as:

$$S - S_f = \frac{-A}{|Q_f|} D \frac{dS}{dx} \quad (2)$$

where: S : Salinity; S_f the fluvial salinity; A is the area of the section; Q_f the fluvial flow and D the longitudinal dispersion coefficient.

Relationship between salinity and dispersion coefficient, based on the work of Van der Burg (1972), defined by:

$$\frac{dD}{dx} = -K \frac{|Q_f|}{A} \quad (3)$$

where K is the Van der Burgh coefficient.

The longitudinal distribution of salinity can be expressed by replacing Equation 2 in 3 and integrating it and considering the fluvial salinity as zero, we have:

$$S_x = S_0 \cdot \left(\frac{D_x}{D_0} \right)^{\frac{1}{K}} \quad (4)$$

where S_x is the salinity at a point in the estuary (psu); S_0 the salinity at the mouth of the estuary (psu); D_x the coefficient of longitudinal dispersion at a point in the estuary ($m^2 \cdot s^{-1}$) and D_0 the coefficient of longitudinal dispersion at the mouth of the estuary ($m^2 \cdot s^{-1}$).

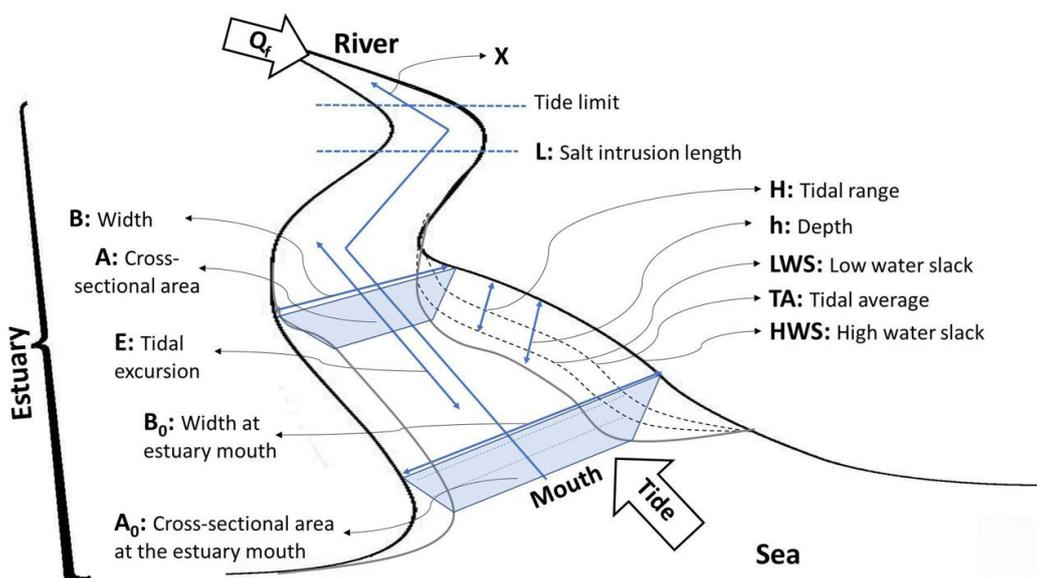


Figure 2. Outline of the definition of the parameters of the saline intrusion model in an alluvial estuary.

In estuaries of alluvial plains the cross-sectional area and the width (Figure 2) can be described by exponential functions along the longitudinal axis of the estuary (Savenije, 2005), according to the following equations:

$$A = A_0 \cdot e^{\left(\frac{-x}{a}\right)} \quad (5)$$

$$B = B_0 \cdot e^{\left(\frac{-x}{b}\right)} \quad (6)$$

where a is the cross-sectional convergence length (m) and b the convergence length of the stream width (m).

Substituting the exponential relationship of Equation 5 in Equation 3 and integrating we have:

$$D_x = D_0 \cdot \left\{ 1 - \beta \left[e^{\left(\frac{x}{a}\right)} - 1 \right] \right\} \quad (7)$$

$$\text{Being: } \beta = \frac{K \cdot a}{\alpha_0 \cdot A_0} \quad (8)$$

$$\alpha_0 = \frac{-D_0}{Q_f} \quad (9)$$

where β : rate of dispersion reduction rate, adimensional and α_0 : coefficient of the mixture in the mouth of the estuary in (m^{-1}).

When the salinity of a given point in the estuary (S_x) is equal to the fluvial salinity (S_f), the longitudinal dispersion coefficient will be equal to zero ($D_x = 0$), rearranging Equation 7, we can estimate the salt intrusion length (L).

$$L = a \cdot \ln\left(\frac{1}{\beta} + 1\right) \quad (10)$$

In the calibration process, the parameters that can be adjusted are the Van der Burgh coefficient (K) and the parameter α_0 , in the condition of high water slack. The Van der Burgh coefficient was estimated according to Gisen et al. (2015a).

The longitudinal dispersion coefficient at the mouth of the estuary (D_0) is an essential parameter for the model and there is no way to measure it directly, being calculated for the condition of high water slack, according to Deynoot, (2011).

$$D_0^{HWS} = 1400 \cdot \frac{h_0 \cdot E^{0.5}}{a} \cdot \sqrt{\frac{\Delta\rho}{\rho} \cdot g \cdot \frac{Q_f \cdot T}{B_0}} \quad (11)$$

where D_0^{HWS} is the longitudinal dispersion coefficient at the estuary mouth in the high water slack ($\text{m}^2 \cdot \text{s}^{-1}$); h_0 is the depth of the estuary at the mouth (1.6 m); E is tidal excursion (2,742.69 m); ρ the density ($\text{kg} \cdot \text{m}^{-3}$); $\Delta\rho$ the difference in density; g the acceleration of gravity ($\text{m} \cdot \text{s}^{-2}$) and T the tidal period (43082 s).

After calibration of the model in the high water slack condition, the D_0 is estimated for average tide and low water slack, using Equations 13 and 14.

$$D_0^{TA} = D^{HWS} \left(\frac{E}{2}\right) e^{\left(\frac{-E}{2 \cdot a}\right)} \quad (12)$$

$$D_0^{LWS} = D^{HWS} (E) \cdot e^{\left(\frac{-E}{a}\right)} \quad (13)$$

where D_0^{TA} is the longitudinal dispersion coefficient at the mouth of the estuary at tidal average ($\text{m}^2 \cdot \text{s}^{-1}$) and D_0^{LWS} the longitudinal dispersion coefficient at the mouth of the estuary at low water slack ($\text{m}^2 \cdot \text{s}^{-1}$).

Analysis of the saline intrusion model

The steps of calibration, verification, sensitivity analysis, and validation of the model are fundamental for the consistency between the results predicted by the model and the actual values of the system studied (Fleck et al., 2013). For the model calibration, data obtained on the day, August 24, 2015, in high tide condition, was used, the calculated K (0.43) was kept fixed and the parameter α_0 (Equation 9) was adjusted, varying the calculated value upwards and downwards until the mean square errors (RMSE), as calculated according to Equation 14, were reduced by maximum. The normality of the distribution of errors and their homoscedasticity were evaluated by the Shapiro-Wilk test ($p > 0.05$) and the Breusch-Pagan test ($p > 0.05$), respectively. The index of agreement (dr) used, proposed by Willmott et al. (2011), is expressed in Equation 15.

$$RMSE = \sqrt{\frac{\sum (Y_{est} - Y_{obs})^2}{n}} \quad (14)$$

$$dr = 1 - \frac{\sum |Y_{est} - Y_{obs}|}{2 \cdot \sum |Y_{obs} - Y_{obs_{medio}}|} \quad (15)$$

where $RMSE$ is the mean root of the quadratic errors; Y_{est} the values estimated by the model; Y_{obs} the values observed in the field; n the number of observations; $Y_{obs_{medio}}$ the mean of the values observed in the field and dr the index of agreement.

In a sensitivity analysis, the model response was tested using variations of 30% in the calibrated value of the parameter α_0 and variations of 50% in the calculated Van der Burgh (K) coefficient. In the model validation, water salinity data was used at three times of the tide on different days. The average tidal heights at the time of these samplings, for validation, were 22/01/14 low tide (0.3 m), 23/10/13; 06/12/13 and 16/04/14 average tide (0.5 m); high tide of 14/02/14 (0.9 m).

Scenarios

Law 14,975/09 of the State of Santa Catarina establishes conditions for the discharge of effluents into estuaries and other water bodies, where all assessments must be made for the most unfavorable conditions to the environment (maximum effluent flow) to order to ensure quality standards foresee for the body of water (art. 177, VII). Based on the legal prerogative, three

scenarios were applied to the calibrated model: I) distribution of salinity in the Papaquara estuary without the contribution of anthropic flow; II) distribution of salinity with the flow for which the WWTP was designed, i.e. $285 \text{ L}\cdot\text{s}^{-1}$; III) distribution of salinity with the WWTP under a maximum hourly flow or peak design flow, of $495 \text{ L}\cdot\text{s}^{-1}$.

RESULTS

Water quality

The average concentration of BOD observed at the WWTP outlet was $35.5 \text{ mg}\cdot\text{L}^{-1}$, with variations between minimum and maximum of 2.8 and $137 \text{ mg}\cdot\text{L}^{-1}$, respectively. The mean total phosphorus concentration in the effluent of the WWTP was $3.0 \text{ mg}\cdot\text{L}^{-1}$, varying between 0.9 and $6.9 \text{ mg}\cdot\text{L}^{-1}$ (Figure 3). The concentrations of TP and BOD in P5 (downstream of WWTP, Figure 1) present the mean values of 0.8 and $7.0 \text{ mg}\cdot\text{L}^{-1}$,

respectively. In the same P5, the lowest DO% values were observed (Figure 3), with a mean value of 3.5% saturation. Low levels of DO were quantified in the whole estuary, being 55% of the samples characterized as hypoxic ($< 2 \text{ mg}\cdot\text{L}^{-1}$, or approximately 30% saturation, Rabalais et al., 2010) and 20% of them with anoxic ($N = 63$).

The concentrations of TP and DO% showed a significant linear correlation with salinity downstream of WWTP (P5 until P1), as Figure 4.

The average flow observed at the WWTP was $143 \text{ L}\cdot\text{s}^{-1}$, with variations between minimum and maximum of 68 and $187 \text{ L}\cdot\text{s}^{-1}$, respectively. The BOD load increased elevenfold from upstream to downstream, from 0.18 to $1.98 \text{ gO}_2\cdot\text{s}^{-1}$ and the total phosphorus load increased fifteenfold, from 0.016 upstream to $0.239 \text{ gP}\cdot\text{s}^{-1}$ downstream (Figure 5).

Considering the average flow and concentration of the final effluent of the WWTP in terms of DBO and TP, it was possible to calculate the current and future load of the WWTP in scenarios II and III (flow of $285 \text{ L}\cdot\text{s}^{-1}$ and $495 \text{ L}\cdot\text{s}^{-1}$), according

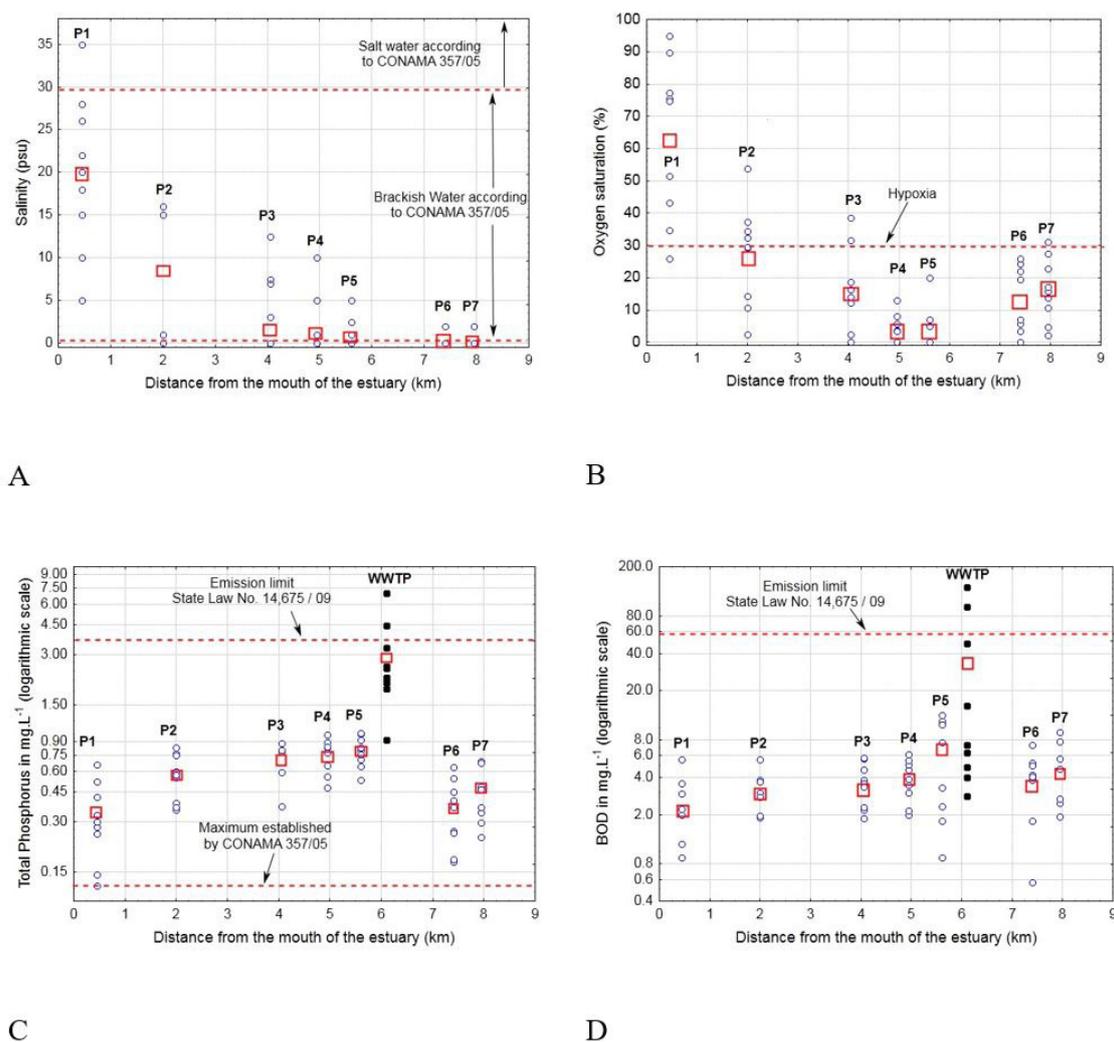
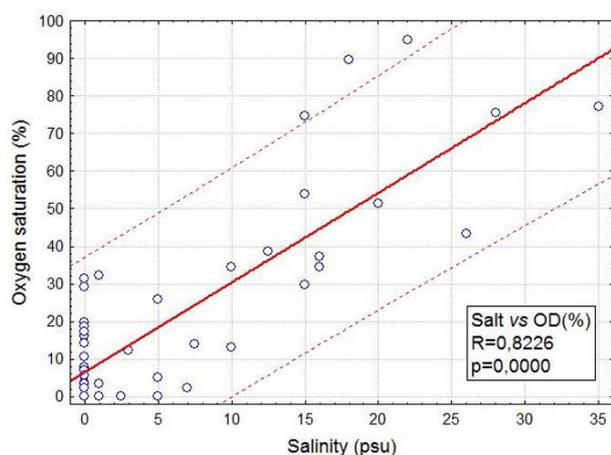
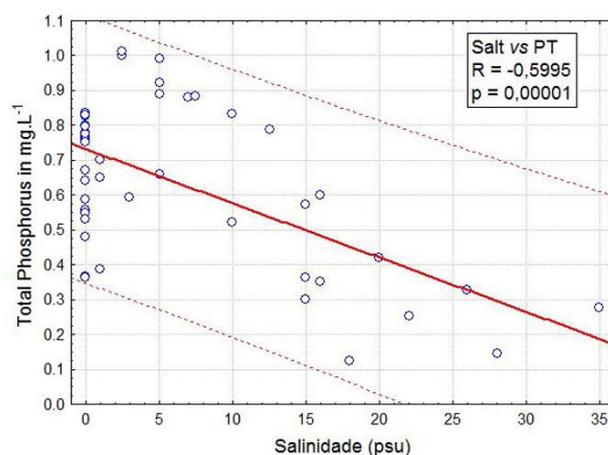


Figure 3. Dispersion chart of the parameters: salinity (A); the percentage of oxygen saturation (B); total phosphorus (C) and BOD (D) in the waters of the Papaquara River and the effluent treated by WWTP. Black points represent the concentration in the final effluent of the WWTP, empty points the concentration in the Papaquara River, red squares correspond to the average values of the sample points.

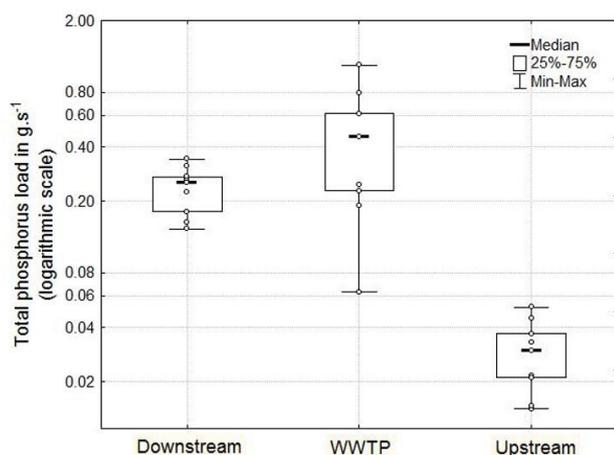


A

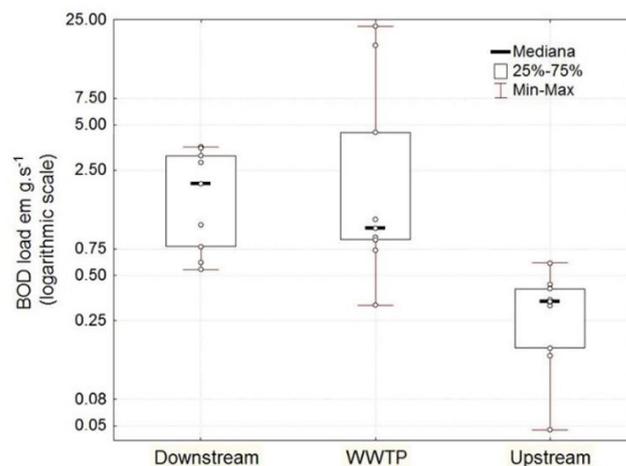
Figure 4. Linear regression between DO% salinity and TP with a 95% prediction interval and correlation coefficient. Oxygen Saturation and Salinity (A) and Total Phosphorus and Salinity (B).



B



A



B

Figure 5. Box Plot, median, upper and lower quartiles, and maximum-minimum values of total phosphorus loads (A) and DBO (B), at WWTP and points upstream and downstream of WWTP on Papaquara River.

to Table 1. For peak flow, maximum hourly flow, scenario III, the DBO and TP load of WWTP can increase by 224% and 219%, respectively, concerning the current load (Table 1).

Saline intrusion

During the calibration of the saline intrusion model, the α_0 parameter was adjusted and used the WWTP flow rate of the data acquisition period, $85 \text{ L}\cdot\text{s}^{-1}$ and Q_{50} of river ($496 \text{ L}\cdot\text{s}^{-1}$) the diffuse flow of the population not served by the sewage system was not considered. The value calculated for the parameter α_0 was 370 m^{-1} (Equation 9), resulting in an RMSE of 2.4. In the

calibration process, we tested values of α_0 between 220 and 500 m^{-1} , with intervals of 5 m^{-1} , being observed the lowest value of RMSE (1.1) with α_0 of 295 m^{-1} , (Figure 6).

The residuals of the calibrated model showed normal distribution and homoscedasticity ($p > 0.05$). The good performance of the model is demonstrated by the high index of agreement (dr) which was 0.85. In the sensitivity test the values of α_0 used were 206 and 383 (30% of the calibrated value) and of the parameters K 0.21 and 0.64 (50% of 0.43, value calculated), as shown in Figure 7. The sensitivity test suggests that the model is more sensitive to the parameter α_0 than the coefficient K (Figure 7). In the validation, the same flow of the calibration was used. The model was consistent during the validation, as observed in

Figure 8, obtaining high indexes of agreement (dr), which varied between 0.84 and 0.92.

The saline intrusion model developed by Savenije (1986, 2005) has been applied in estuaries with very distinct physical characteristics of the Papaquara River estuary. The data available in Savenije (2020), for 35 estuaries out of 86 collections, demonstrate that the model has already been applied in environments with river flow rates ranging from 2,000 to 316,000 L·s⁻¹, mixing coefficient at the estuary mouth (α_0), ranging from 0.13 to 48 m⁻¹, and saline intrusion at high tide (L^{HWS}) between 10 to 160 km. The flow of Papaquara presents average fluvial flows of 496 L·s⁻¹, mixing coefficient of 295 m⁻¹, and saline intrusion at a high tide of 9 km. This model has been applied for the description of saline intrusion in estuaries (Ervin et al., 2007; Zhang et al., 2011;

Gisen et al., 2015b), determination of freshwater discharge in estuaries (Nguyen et al., 2008) and in the prediction of increased intrusion due consumptive use of water (Cai et al., 2015; Abdullah, 2017). Even facing these differences, the model proved to be robust, with coherent results such as those observed under field conditions (Figure 8), thus enabling reliable forecasts. However, further studies should be carried out, with the objective of evaluating the effect of daily fluctuations in the flow of WWTP in the saline intrusion of Papaquara, considering that the flow of WWTP is of the same order of magnitude as the river flow. The model developed by Savenije (1986, 2005), besides being easy to implement (executable in electronic spreadsheets), requires few input data and proved to be very useful to understand the estuarine functioning and to predict changes in the saline gradient, facing changes in the freshwater input.

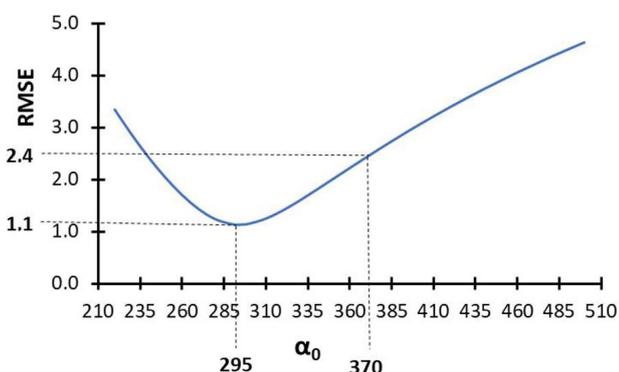
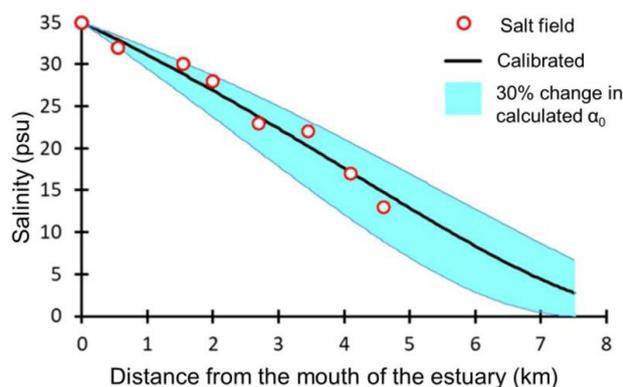


Figure 6. RMSE result during the calibration process of the model, highlighting the RMSE values with α_0 calculated and calibrated.

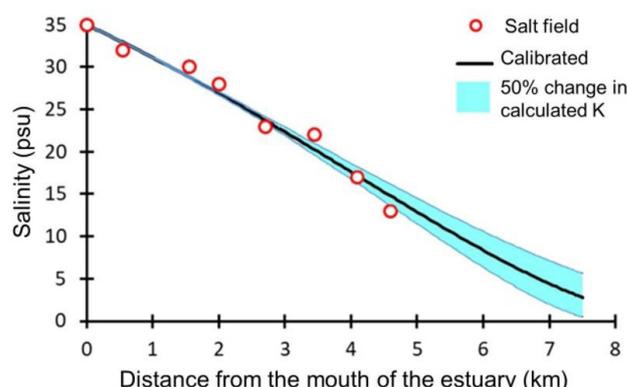
Scenarios

From the proposed scenarios, a progressive reduction of the saline gradient was observed with the increase of anthropic flow (Figure 9). The reduction of saline intrusion in different scenarios varied between 11 and 28%, as shown in Table 2.

The average design capacity flow and the maximum hourly flow (peak flow) of the WWTP could raise the river flow of the estuary by 57 and 100% respectively. In scenarios II and III, of 781 and 991 L·s⁻¹, respectively, this flow can also be expected under natural conditions, but with a low probability of occurrence (Q_{20} and Q_{10}). However, the contribution of treated effluent by the WWTP has as main feature, the continuous discharge in the estuary.



A



B

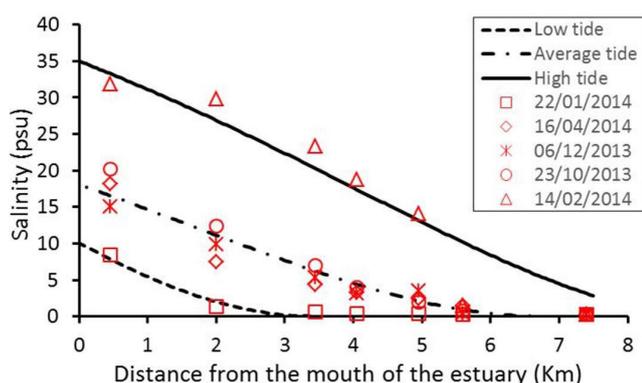
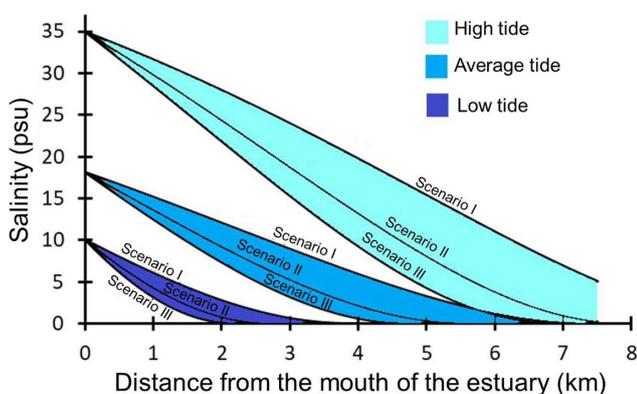
Figure 7. Model sensitivity to variations of parameters α_0 (A) and the coefficient K (B).

Table 1. Load of the current WWTP and the estimated one when the WWTP reaches the projected flows.

Type of flow from WWTP	Flow (L·s ⁻¹)	Load DBO (gO ₂ ·s ⁻¹)	Load PT (gP·s ⁻¹)
Actual (mean)	143	5.42	0.46
Mean Projected (scenario II)	285	10.13	0.84
Maximum Projected (scenario III)	495	17.59	1.47

Table 2. Current saline intrusion and the three different scenarios in km and their modifications in percentage terms related to the current condition.

Condition	Current	Scenario I Q_{WWTP} :		Scenario II Q_{WWTP} :		Scenario III Q_{WWTP} :	
	$143 \text{ L}\cdot\text{s}^{-1}$	$0 \text{ L}\cdot\text{s}^{-1}$		$285 \text{ L}\cdot\text{s}^{-1}$		$495 \text{ L}\cdot\text{s}^{-1}$	
High Tide	9.0 km	10.2 km	13%	8.0 km	-11%	7.0 km	-22%
Average Tide	6.5 km	7.5 km	15%	5.7 km	-12%	4.8 km	-26%
Low Tide	3.2 km	3.9 km	22%	2.7 km	-16%	2.3 km	-28%

**Figure 8.** Curves resulting from the salinity distribution model in high tide, average tide and low tide conditions; associated with the validation of the model with field data other than those used in calibration.**Figure 9.** Salinity distribution in the three different scenarios and tidal conditions.

DISCUSSION

An increase of the anthropic effluent loaded projected from WWTP in the Papaquara River will decrease the saline intrusion, concomitantly, will increase the negative effect from pollutants. The reduction in saline intrusion (see Figure 9 and Table 2) may lead to changes in the physical dynamics (sediment flow), chemical (contribution of nutrients and organic matter), and biological (distribution and survival of species) of the estuaries (Kimmerer, 2002; Montagna et al., 2003; Bussell et al., 2008; Gillson, 2011). This reduction affects the occurrence of saline flocculation in upstream regions of the estuary. Saline flocculation is an important phenomenon in the self-depuration process of

estuarine waters, adsorbing dissolved substances (Stumm & Morgan, 1996; Bianchi, 2007; Wolanski & Elliott, 2015). Perez et al. (2017) report that after the entry of the effluent from the WWTP into the Papaquara River, there was an increase in the total phosphorus, with predominance of the dissolved phosphorus fraction (80%) in low salinity regions, and along the saline gradient, there was a reduction in total phosphorus concentration and a greater balance between dissolved and particulate phosphorus fractions (approximately 50%). The authors attributed the increase in the fraction of particulate phosphorus to the adsorption processes during saline flocculation and in the resuspension of sediments by the movement of tides. Saline waters of the North Florianopolis Bay have higher oxygen concentrations and lower organic matter (BOD) and nutrient (phosphorus and nitrogen) concentrations when compared to the Papaquara River waters (Simonassi et al., 2010; Silva et al., 2016; Cabral et al., 2020). The inputs of these saline waters improve the water quality along the saline gradient of the estuary, as we can see in Figures 3 and 4, through adsorption and dilution processes. The reduction of saline intrusion indicates the decrease of saline water inflows in more upstream regions of the estuary (Table 2), affecting the self-depuration processes in these regions.

The CONAMA 357/05 resolution classifies waters according to their uses and typifies them according to their salinity, establishing maximum allowed concentrations of several parameters for each class and type. The modification of salinity in the estuary foreseen in this study will cause, besides the environmental effects, legal repercussions on the maximum allowed concentration of water quality parameters. Currently, the waters of the Papaquara River are provisionally classified as brackish class 1 and freshwater class 2 (resolutions CERH 01/08 and CONAMA 357/05 art. 42). According to the results of the model, currently, at high tide, the limit between these types of waters occurs 8.6 km upstream of the mouth of the estuary. In scenarios II and III this limit will be reduced to 7.3 and 6.3 km (Figure 9), that is, there will be a change in the classification of this stretch of the river, from brackish class 1 to freshwater class 2. This stretch is where the entrance of the WWTP effluent occurs (Figure 1). In freshwater class 2, the maximum concentrations allowed for nitrogen compounds (nitrite, nitrate, ammonia) are higher than those allowed in brackish class 1 (CONAMA resolution 357/05 art. 15 and 22). Silva et al. (2016) registered average concentrations of ammonia nitrogen of $1.8 \text{ mg}\cdot\text{L}^{-1}$ in brackish waters at 7.4 km from the mouth of the Papaquara River estuary, the maximum value allowed by the legislation being $0.4 \text{ mg}\cdot\text{L}^{-1}$, configuring the disrespect to the standard. However, if this stretch is reclassified as freshwater class 2, the maximum value allowed would be $3.7 \text{ mg}\cdot\text{L}^{-1}$, considering the pH observed in the stretch by Silva (2015) and Rodrigues

(2016), because for fresh waters, CONAMA 357/05 establishes the maximum concentration of ammoniacal nitrogen as a function of pH. This change will further weaken the management of this water body that drains into a protected area. Disposal of treated effluents is discussed in the city through a working group with the participation of institutions at the Municipal, State and Federal levels (Municipal Decree n. 17,748/2017). For this group, water bodies that have a strong relationship with aquatic environments of protected areas of category full protection (Law 9,985/00 art. 8) should be classified as special class (Florianópolis, 2019), which would make the disposal of effluents unfeasible, even for treated disposals (CONAMA 430/11 art. 11). This is the situation of the Papaquara River, which flows to Ecological Station of Carijós, protected area of full protection type, and which receives effluents from WWTP (Figure 1), where its waters do not meet the quality standards by legislation (Figure 3).

The Brazilian environmental legislation prohibits that the entrance of effluents in a watercourse that alters its quality characteristics established for its class (CONAMA 430/11, art. 5). Despite the prohibition, the reality of rivers along the Brazilian coast shows that a load of organic matter, mainly from domestic sewage, is high and is promoting the degradation of estuaries (Aguiar et al., 2011; Souza & Gastaldini, 2014; Cabral et al., 2020) and, consequently, violation of the standard. The situation in the Papaquara River is not different, the high concentrations of organic matter and nutrients, in conjunction to low concentrations of OD are at disagreement with the parameters established for its class (Figure 3). In the present study, 80% of the WWTP effluent samples presented BOD and TP concentrations below the maximum emission limit established by the Santa Catarina Law n° 14 675/09 (art. 177, inc. V and XI). However, more than 90% of the dissolved oxygen and total phosphorus data observed in the Papaquara River are in disagreement with the legislation (Figure 3). The legislation for water body (CONAMA 357/05, art. 21), establishes a minimum oxygen value of 5 mg·L⁻¹ (approximately 60-70% of saturation) and for the total phosphorus of 0.124 mg·L⁻¹. This paradoxical picture of compliance with emission standards and low quality of receiving water body can be explained by the reduced carrying capacity of the Papaquara River, which is understood as the capacity of the water body to receive a polluting load without compromising the quality of its waters, according to its framework (CONAMA 430/11 art. 4, inc. I). This reduced carrying capacity becomes evident when comparing the upstream and downstream loads of the WWTP (Figure 5). Even if the quantified parameter concentrations in the effluent are below the maximum emission limit established by the Law (Figure 3), the volume launched is such that it exceeds the estuary's self-purification capacity, via biogeochemical (assimilation and adsorption) and physical (dilution) processes (Bianchi, 2007). With the increase in the load of the WWTP (Table 1), there will be an expansion of the current areas of the estuary that already suffer from hypoxia to anoxia (Figure 3B), impacting the functioning and uses of this environment. Poor water quality is been observed in other water bodies in the city even area covered by wastewater collection and treatment systems (Garbossa et al., 2017; Cabral et al., 2019). To avoid aggravating the situation, the working group established by City Decree n. 17,748/17, has been looking for alternatives for

the final disposal of effluents generated in the city, considering alternatives such as submarine outfalls and indirect reuse through the recharge of aquifers (Florianópolis, 2019).

The European Community legislation does not establish fixed water quality values for estuarine environments, but guides a study of the typology of environments in each country and also establishes that the levels of effluent treatment must be defined according to the current quality of the waters of the receiving body and the anthropic pressure that the water body is submitted to (Silva et al., 2018). The role of WWTP in preventing and even reversing pollution in several coastal systems is notorious (Morrison et al., 2011; Cloern et al., 2016), since well operated and designed (Cabral et al., 2019), and, especially, that consider the carrying capacity of the body receiving the treated effluents. Thus, the evaluation of effluent discharged by the WWTP should be based more on the carrying capacity of the receiving water body (CONAMA 430/11, Art. 5) than on the maximum emission values.

CONCLUSION

With the increase in the flow and organic load launched by WWTP, there will be a reduction in saline intrusion and increase in the areas in hypoxic and anoxic conditions, confirming the hypothesis initially raised. This change will negative impacts for the local biota and biogeochemistry process in regions further up the estuary. For the WWTP to bring the expected environmental gains, it is more important to consider the carrying capacity of the receiving body than to the maximum emission values established.

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Authors contributions

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